



ENGINEERING PLASTICITY I

Jiří Plešek

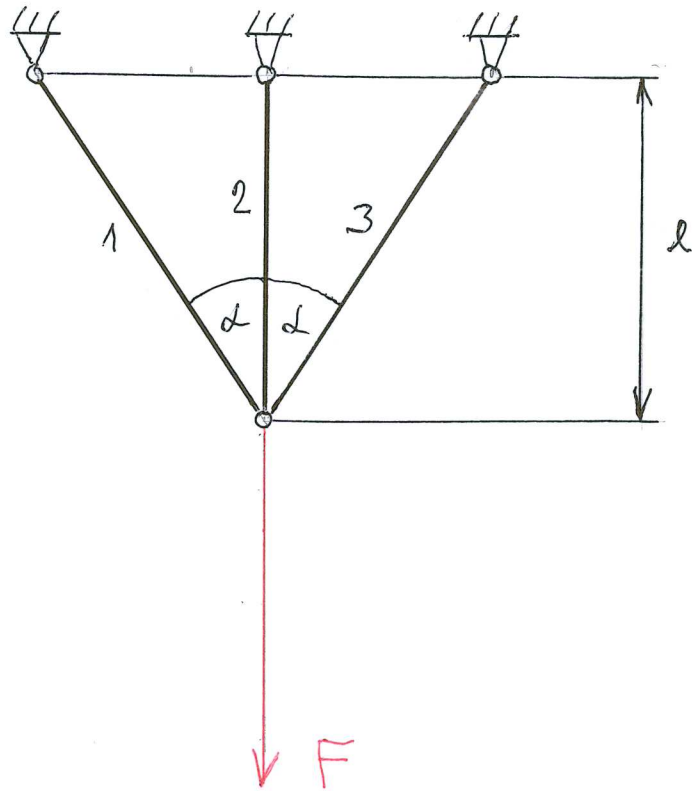
Institute of Thermomechanics
Czech Academy of Sciences



Contents

- Brittle and ductile fracture
- Elastic and plastic limits
- Static load analysis
- Residual stresses and shakedown analysis
- High and low cycle fatigue

Truss structure

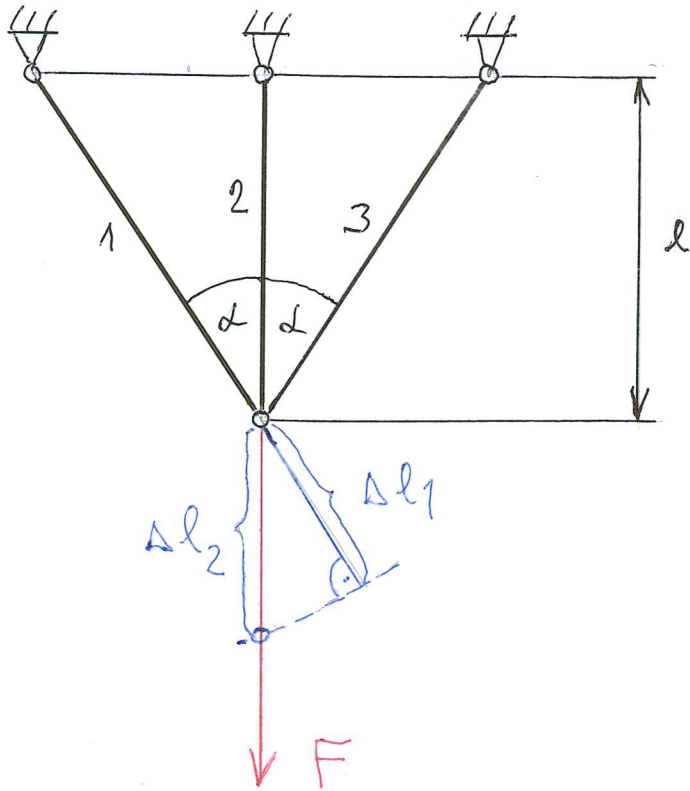


equilibrium

$$2N_1 \cos \alpha + N_2 = F$$

Load carrying capacity?

Truss structure



equilibrium

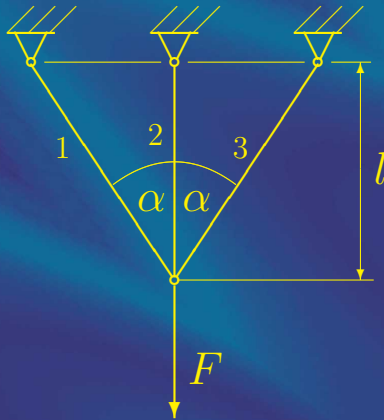
$$2N_1 \cos \alpha + N_2 = F$$

compatibility

$$\Delta l_2 \cos \alpha = \Delta l_1$$

Load carrying capacity?

Elastic solution (1/2)



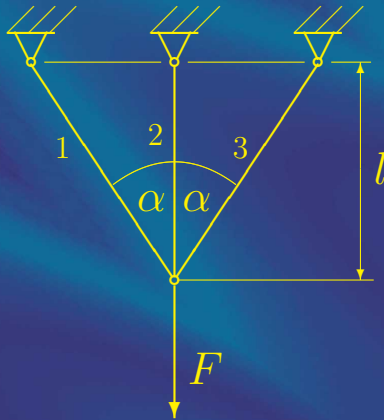
equilibrium

$$2N_1 \cos \alpha + N_2 = F$$

compatibility

$$\Delta l_2 \cos \alpha = \Delta l_1$$

Elastic solution (1/2)



equilibrium

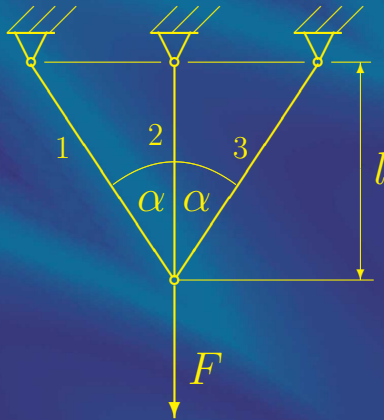
$$2N_1 \cos \alpha + N_2 = F$$

compatibility

$$\Delta l_2 \cos \alpha = \Delta l_1$$

Hooke: $\Delta l_2 = \frac{N_2 l}{EA}$, $\Delta l_1 = \frac{N_1 l}{EA} \frac{1}{\cos \alpha}$

Elastic solution (1/2)



equilibrium

$$2N_1 \cos \alpha + N_2 = F$$

compatibility

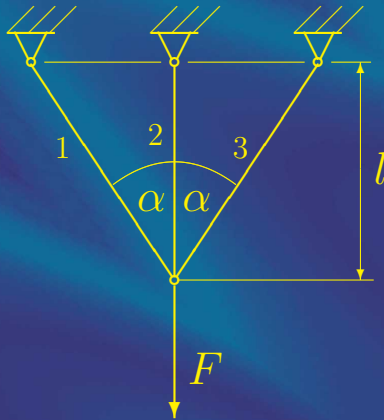
$$\Delta l_2 \cos \alpha = \Delta l_1$$

Hooke: $\Delta l_2 = \frac{N_2 l}{EA}$, $\Delta l_1 = \frac{N_1 l}{EA} \frac{1}{\cos \alpha}$

$$N_2 \cos^2 \alpha = N_1$$

(Beltrami-Michell)

Elastic solution (1/2)



equilibrium

$$2N_1 \cos \alpha + N_2 = F$$

compatibility

$$\Delta l_2 \cos \alpha = \Delta l_1$$

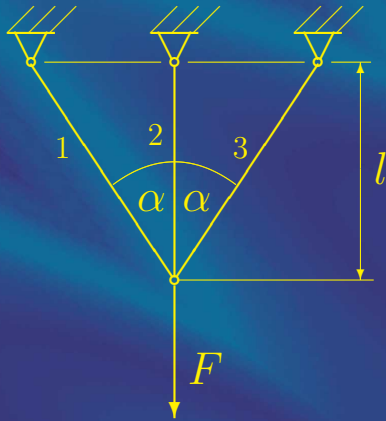
Hooke: $\Delta l_2 = \frac{N_2 l}{EA}$, $\Delta l_1 = \frac{N_1 l}{EA} \frac{1}{\cos \alpha}$

$$N_2 \cos^2 \alpha = N_1$$

(Beltrami-Michell)

Question: Equilibrium versus BM equation?

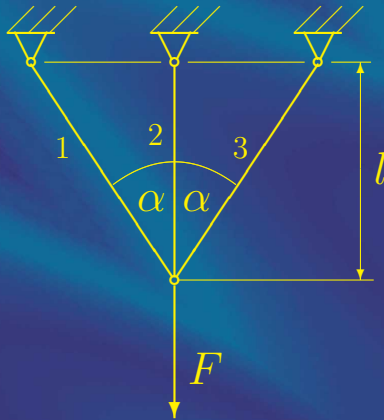
Elastic solution (2/2)



$$N_1 = \frac{F \cos^2 \alpha}{1 + 2 \cos^3 \alpha}$$

$$N_2 = \frac{F}{1 + 2 \cos^3 \alpha}$$

Elastic solution (2/2)



$$N_1 = \frac{F \cos^2 \alpha}{1 + 2 \cos^3 \alpha}$$

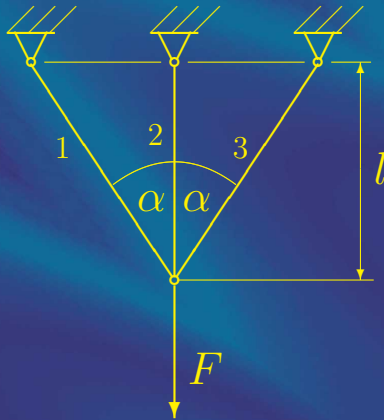
$$N_2 = \frac{F}{1 + 2 \cos^3 \alpha}$$

Critical state

$$N_1 = N_3 < N_2 = N_{\text{crit}} \Rightarrow$$

$$F_{\text{crit}} = N_{\text{crit}}(1 + 2 \cos^3 \alpha)$$

Elastic solution (2/2)



$$N_1 = \frac{F \cos^2 \alpha}{1 + 2 \cos^3 \alpha}$$

$$N_2 = \frac{F}{1 + 2 \cos^3 \alpha}$$

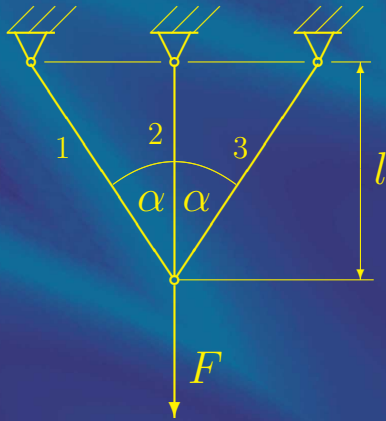
Critical state

$$N_1 = N_3 < N_2 = N_{\text{crit}} \Rightarrow$$

$$F_{\text{crit}} = N_{\text{crit}}(1 + 2 \cos^3 \alpha)$$

Remark: Independence on E , A , l . We speak about a *universal solution*.

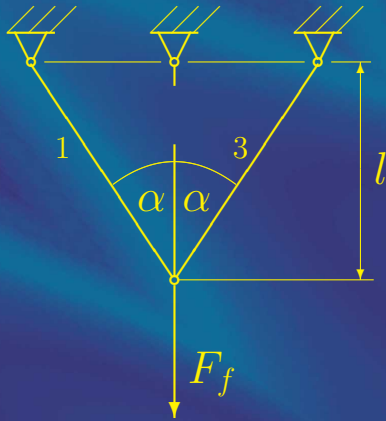
Brittle fracture



Let $N_{\text{crit}} = N_f = \text{force to rupture}$

The middle strut (2) ruptures at
 $F_f = N_f(1 + 2 \cos^3 \alpha)$

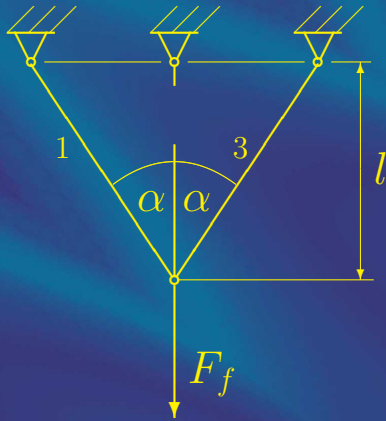
Brittle fracture



Let $N_{\text{crit}} = N_f = \text{force to rupture}$

The middle strut (2) ruptures at
 $F_f = N_f(1 + 2 \cos^3 \alpha)$

Brittle fracture

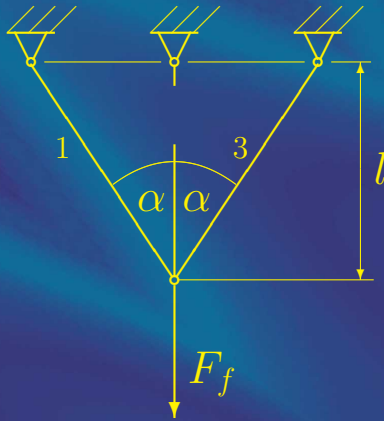


Let $N_{\text{crit}} = N_f = \text{force to rupture}$

The middle strut (2) ruptures at
 $F_f = N_f(1 + 2 \cos^3 \alpha)$

Simplified equilibrium: $2N_1 \cos \alpha + 0 = F_f$

Brittle fracture

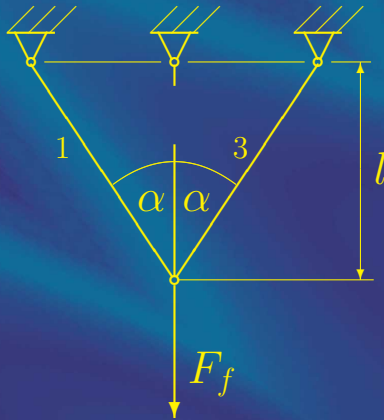


Let $N_{\text{crit}} = N_f = \text{force to rupture}$

The middle strut (2) ruptures at $F_f = N_f(1 + 2 \cos^3 \alpha)$

Simplified equilibrium: $2N_1 \cos \alpha + 0 = F_f \Rightarrow N_1 = N_f \frac{1 + 2 \cos^3 \alpha}{2 \cos \alpha}$

Brittle fracture

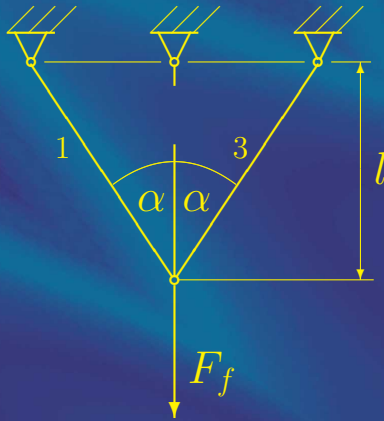


Let $N_{\text{crit}} = N_f = \text{force to rupture}$

The middle strut (2) ruptures at $F_f = N_f(1 + 2 \cos^3 \alpha)$

Simplified equilibrium: $2N_1 \cos \alpha + 0 = F_f \Rightarrow N_1 = N_f \underbrace{\frac{1 + 2 \cos^3 \alpha}{2 \cos \alpha}}_{> 1} > N_f$

Brittle fracture



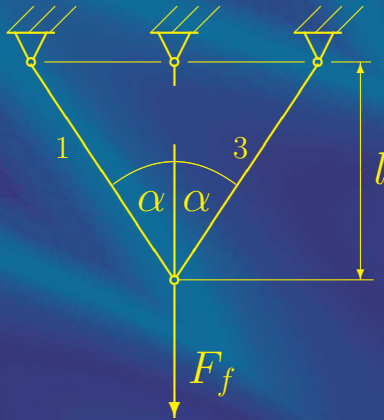
Let $N_{\text{crit}} = N_f = \text{force to rupture}$

The middle strut (2) ruptures at $F_f = N_f(1 + 2 \cos^3 \alpha)$

Simplified equilibrium: $2N_1 \cos \alpha + 0 = F_f \Rightarrow N_1 = N_f \underbrace{\frac{1 + 2 \cos^3 \alpha}{2 \cos \alpha}}_{> 1} > N_f$

$$F < F_f = N_f(1 + 2 \cos^3 \alpha)$$

Brittle fracture



Let $N_{\text{crit}} = N_f = \text{force to rupture}$

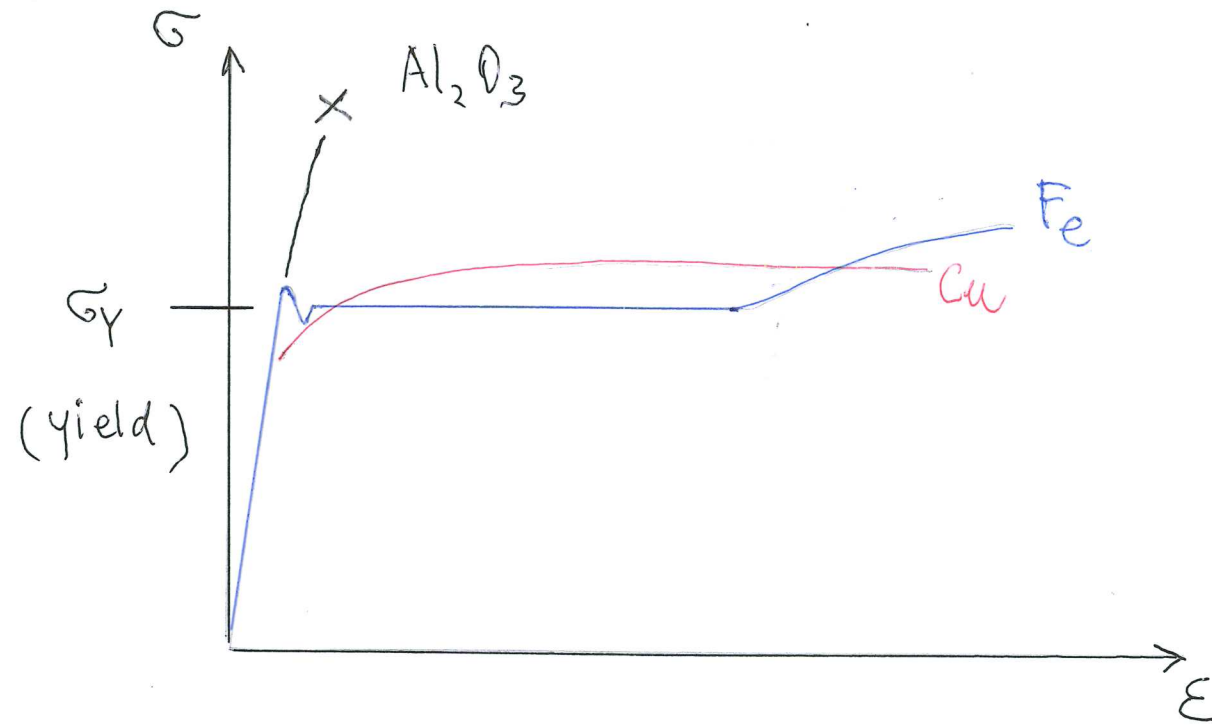
The middle strut (2) ruptures at $F_f = N_f(1 + 2 \cos^3 \alpha)$

Simplified equilibrium: $2N_1 \cos \alpha + 0 = F_f \Rightarrow N_1 = N_f \underbrace{\frac{1 + 2 \cos^3 \alpha}{2 \cos \alpha}}_{> 1} > N_f$

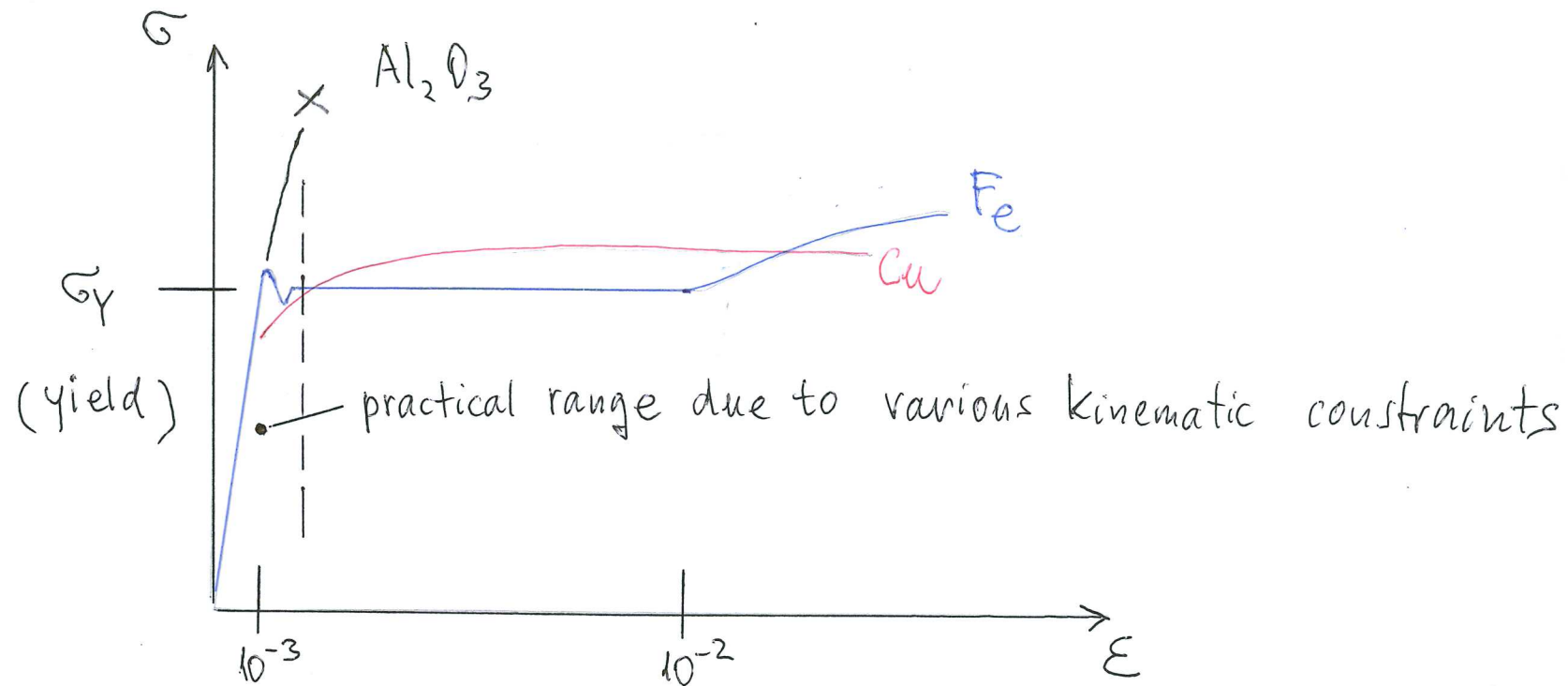
$$F < F_f = N_f(1 + 2 \cos^3 \alpha)$$

Remark: Ignoring (2) altogether a conservative condition follows as $F < 2N_{\text{crit}} \cos \alpha$

Typical stress-strain curves

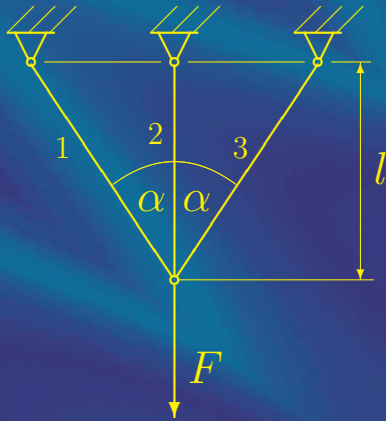


Typical stress-strain curves



Idealization $\sigma = \sigma_Y = \text{const.}$ is good and useful.

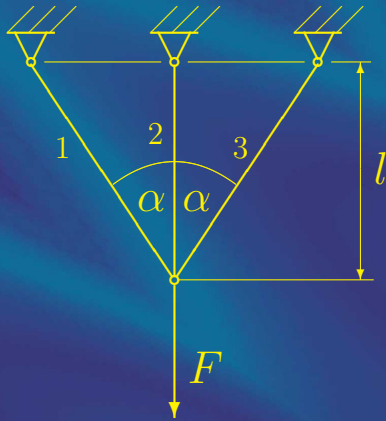
Ductile fracture



Let $N_{\text{crit}} = N_Y = A\sigma_Y$

The middle strut (2) yields at
 $F_e = N_Y(1 + 2\cos^3 \alpha)$

Ductile fracture

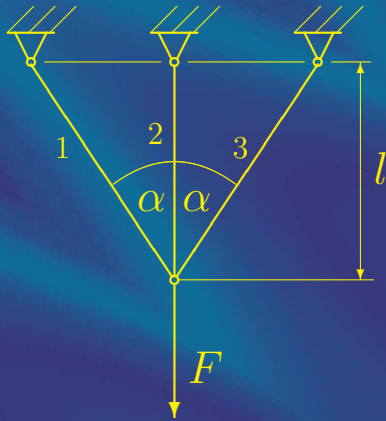


Let $N_{\text{crit}} = N_Y = A\sigma_Y$

The middle strut (2) yields at
 $F_e = N_Y(1 + 2\cos^3 \alpha)$

Simplified equilibrium: $2N_1 \cos \alpha + N_Y = F > F_e$

Ductile fracture

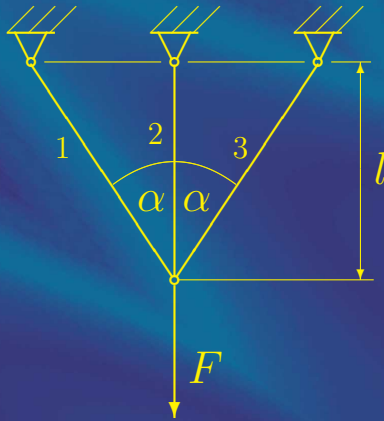


Let $N_{\text{crit}} = N_Y = A\sigma_Y$

The middle strut (2) yields at
 $F_e = N_Y(1 + 2 \cos^3 \alpha)$

Simplified equilibrium: $2N_1 \cos \alpha + N_Y = F > F_e \Rightarrow F_p = N_Y(1 + 2 \cos \alpha)$

Ductile fracture



$$\text{Let } N_{\text{crit}} = N_Y = A\sigma_Y$$

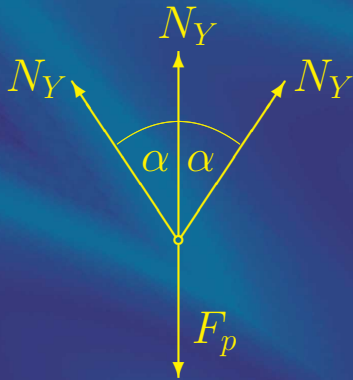
The middle strut (2) yields at
 $F_e = N_Y(1 + 2 \cos^3 \alpha)$

$$\text{Simplified equilibrium: } 2N_1 \cos \alpha + N_Y = F > F_e \Rightarrow F_p = N_Y(1 + 2 \cos \alpha)$$

$$F < F_p = N_Y(1 + 2 \cos \alpha)$$

Remark: No elastic solution needed!

Plastic limit analysis



Admissible force (stress) method

$$2N_Y \cos \alpha + N_Y = F_p$$

$$F_p = N_Y(1 + 2 \cos \alpha)$$

Remark: Compatibility equation not needed!



Collapse load margin

Elastic-plastic load factor

$$\kappa = \frac{F_p}{F_e}$$



Collapse load margin

Elastic-plastic load factor

$$\kappa = \frac{F_p}{F_e}$$

Truss structure problem

$$\kappa = \frac{1 + 2 \cos \alpha}{1 + 2 \cos^3 \alpha} = 1.2 \quad (\text{for } \alpha = 30^\circ)$$



Collapse load margin

Elastic-plastic load factor

$$\kappa = \frac{F_p}{F_e}$$

Truss structure problem

$$\kappa = \frac{1 + 2 \cos \alpha}{1 + 2 \cos^3 \alpha} = 1.2 \quad (\text{for } \alpha = 30^\circ)$$

Typical value for bending and torsion, $\kappa \simeq 1.5$, is used in many design codes.



Static load assessment

Load factor

$$F < \frac{F_p}{k}, \quad k \simeq 1.5$$



Static load assessment

Load factor

$$F < \frac{F_p}{k}, \quad k \simeq 1.5$$

Factor of safety

$$F < \frac{F_p}{k} = \frac{\kappa F_e}{k}$$



Static load assessment

Load factor

$$F < \frac{F_p}{k}, \quad k \simeq 1.5$$

Factor of safety

$$F < \frac{F_p}{k} = \frac{\kappa F_e}{k} = \frac{F_e}{k'}, \quad k' = \frac{k}{\kappa} \simeq 1$$



Static load assessment

Load factor

$$F < \frac{F_p}{k}, \quad k \simeq 1.5$$

Factor of safety

$$F < \frac{F_p}{k} = \frac{\kappa F_e}{k} = \frac{F_e}{k'}, \quad k' = \frac{k}{\kappa} \simeq 1, \quad \text{we usually set } k' \geq 1$$



Static load assessment

Load factor

$$F < \frac{F_p}{k}, \quad k \simeq 1.5$$

Factor of safety

$$F < \frac{F_p}{k} = \frac{\kappa F_e}{k} = \frac{F_e}{k'}, \quad k' = \frac{k}{\kappa} \simeq 1, \quad \text{we usually set } k' \geq 1$$

Allowable stress

$$\sigma_{\max} = f(F)$$



Static load assessment

Load factor

$$F < \frac{F_p}{k}, \quad k \simeq 1.5$$

Factor of safety

$$F < \frac{F_p}{k} = \frac{\kappa F_e}{k} = \frac{F_e}{k'}, \quad k' = \frac{k}{\kappa} \simeq 1, \quad \text{we usually set } k' \geq 1$$

Allowable stress

$$\sigma_{\max} = f(F) < f\left(\frac{F_e}{k'}\right)$$



Static load assessment

Load factor

$$F < \frac{F_p}{k}, \quad k \simeq 1.5$$

Factor of safety

$$F < \frac{F_p}{k} = \frac{\kappa F_e}{k} = \frac{F_e}{k'}, \quad k' = \frac{k}{\kappa} \simeq 1, \quad \text{we usually set } k' \geq 1$$

Allowable stress

$$\sigma_{\max} = f(F) < f\left(\frac{F_e}{k'}\right) = \frac{1}{k'} f(F_e)$$



Static load assessment

Load factor

$$F < \frac{F_p}{k}, \quad k \simeq 1.5$$

Factor of safety

$$F < \frac{F_p}{k} = \frac{\kappa F_e}{k} = \frac{F_e}{k'}, \quad k' = \frac{k}{\kappa} \simeq 1, \quad \text{we usually set } k' \geq 1$$

Allowable stress

$$\sigma_{\max} = f(F) < f\left(\frac{F_e}{k'}\right) = \frac{1}{k'} \underbrace{f(F_e)}_{\sigma_Y}$$



Static load assessment

Load factor

$$F < \frac{F_p}{k}, \quad k \simeq 1.5$$

Factor of safety

$$F < \frac{F_p}{k} = \frac{\kappa F_e}{k} = \frac{F_e}{k'}, \quad k' = \frac{k}{\kappa} \simeq 1, \quad \text{we usually set } k' \geq 1$$

Allowable stress

$$\sigma_{\max} = f(F) < f\left(\frac{F_e}{k'}\right) = \frac{1}{k'} \underbrace{f(F_e)}_{\sigma_Y} = \frac{\sigma_Y}{k'} \equiv \sigma_D$$



Static load assessment

Load factor

$$F < \frac{F_p}{k}, \quad k \simeq 1.5$$

Factor of safety

$$F < \frac{F_p}{k} = \frac{\kappa F_e}{k} = \frac{F_e}{k'}, \quad k' = \frac{k}{\kappa} \simeq 1, \quad \text{we usually set } k' \geq 1$$

Allowable stress

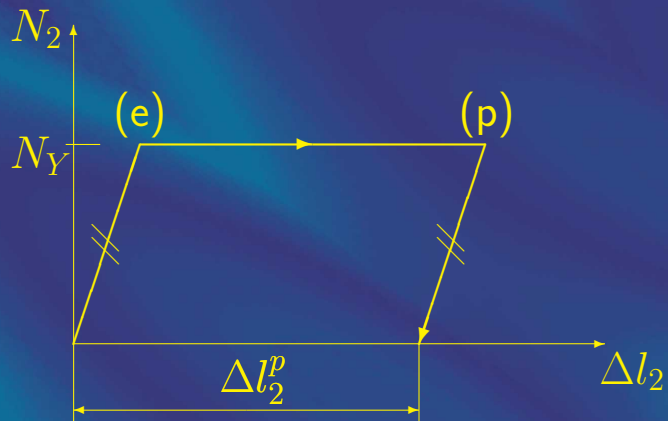
$$\sigma_{\max} = f(F) < f\left(\frac{F_e}{k'}\right) = \frac{1}{k'} \underbrace{f(F_e)}_{\sigma_Y} = \frac{\sigma_Y}{k'} \equiv \sigma_D$$

Elastic analysis only needs to be performed! Plasticity included via FoS.



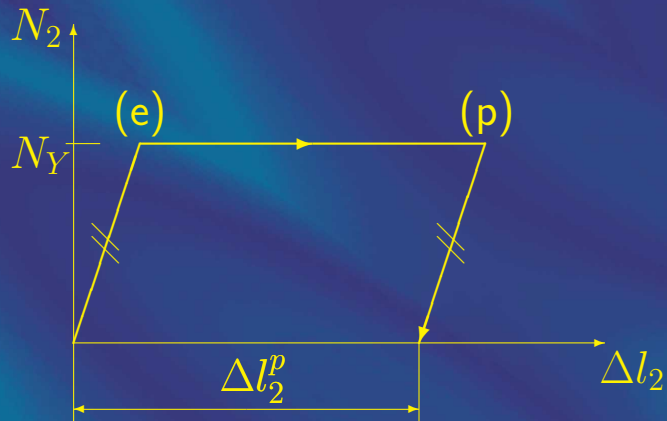
Unloading

Both (1) and (3) load and unload elastically.

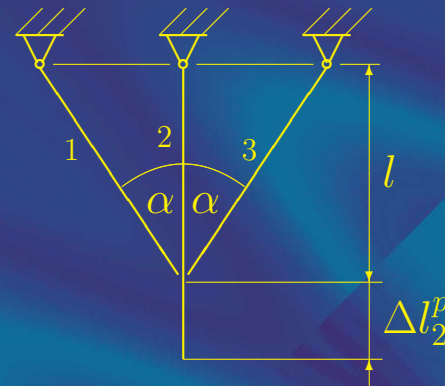


Unloading

Both (1) and (3) load and unload elastically.

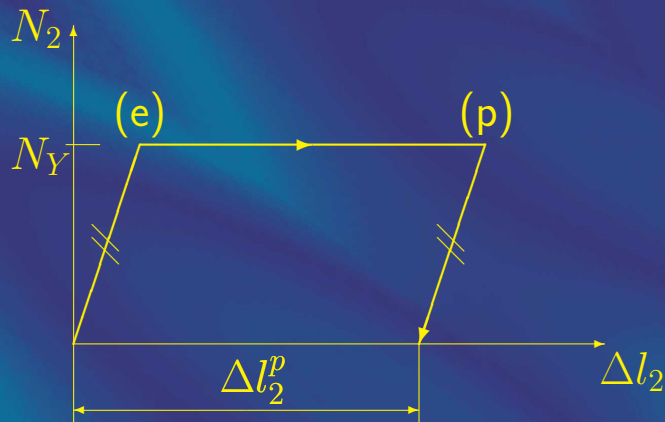


Unloaded and dismantled

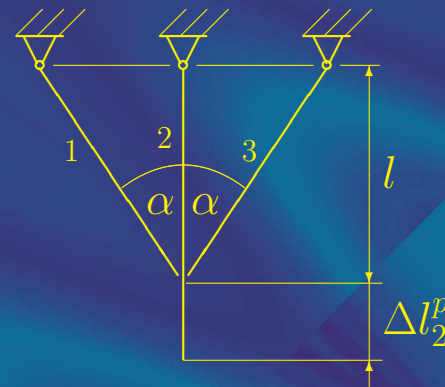


Unloading

Both (1) and (3) load and unload elastically.



Unloaded and dismantled



Residual stresses by solving a statically indeterminate problem or ...



Residual stresses

Elastic solution

$$N_2^{\text{el}}(F) = \frac{F}{1 + 2 \cos^3 \alpha} \quad (\text{linear})$$



Residual stresses

Elastic solution

$$N_2^{\text{el}}(F) = \frac{F}{1 + 2 \cos^3 \alpha} \quad (\text{linear})$$

Define

$$N_2(F) = \begin{cases} N_2^{\text{el}}(F), & F \leq F_e \\ N_Y, & F > F_e \end{cases}$$



Residual stresses

Elastic solution

$$N_2^{\text{el}}(F) = \frac{F}{1 + 2 \cos^3 \alpha} \quad (\text{linear})$$

Define

$$N_2(F) = \begin{cases} N_2^{\text{el}}(F), & F \leq F_e \\ N_Y, & F > F_e \end{cases}$$

Residual force

$$N_2^{\text{res}} = N_2(F) + N_2^{\text{el}}(-F)$$



Residual stresses

Elastic solution

$$N_2^{\text{el}}(F) = \frac{F}{1 + 2 \cos^3 \alpha} \quad (\text{linear})$$

Define

$$N_2(F) = \begin{cases} N_2^{\text{el}}(F), & F \leq F_e \\ N_Y, & F > F_e \end{cases}$$

Residual force

$$N_2^{\text{res}} = N_2(F) + N_2^{\text{el}}(-F) = N_2(F) - N_2^{\text{el}}(F), \quad (\text{as long as } N_2^{\text{res}} \geq -N_Y)$$



Residual stresses

Elastic solution

$$N_2^{\text{el}}(F) = \frac{F}{1 + 2 \cos^3 \alpha} \quad (\text{linear})$$

Define

$$N_2(F) = \begin{cases} N_2^{\text{el}}(F), & F \leq F_e \\ N_Y, & F > F_e \end{cases}$$

Residual force

$$N_2^{\text{res}} = N_2(F) + N_2^{\text{el}}(-F) = N_2(F) - N_2^{\text{el}}(F), \quad (\text{as long as } N_2^{\text{res}} \geq -N_Y)$$

Limit state

$$N_2^{\text{res}} = N_2(F_p) - N_2^{\text{el}}(F_p)$$



Residual stresses

Elastic solution

$$N_2^{\text{el}}(F) = \frac{F}{1 + 2 \cos^3 \alpha} \quad (\text{linear})$$

Define

$$N_2(F) = \begin{cases} N_2^{\text{el}}(F), & F \leq F_e \\ N_Y, & F > F_e \end{cases}$$

Residual force

$$N_2^{\text{res}} = N_2(F) + N_2^{\text{el}}(-F) = N_2(F) - N_2^{\text{el}}(F), \quad (\text{as long as } N_2^{\text{res}} \geq -N_Y)$$

Limit state

$$N_2^{\text{res}} = N_2(F_p) - N_2^{\text{el}}(F_p) = N_2(F_p) - \kappa N_2^{\text{el}}(F_e)$$



Residual stresses

Elastic solution

$$N_2^{\text{el}}(F) = \frac{F}{1 + 2 \cos^3 \alpha} \quad (\text{linear})$$

Define

$$N_2(F) = \begin{cases} N_2^{\text{el}}(F), & F \leq F_e \\ N_Y, & F > F_e \end{cases}$$

Residual force

$$N_2^{\text{res}} = N_2(F) + N_2^{\text{el}}(-F) = N_2(F) - N_2^{\text{el}}(F), \quad (\text{as long as } N_2^{\text{res}} \geq -N_Y)$$

Limit state

$$N_2^{\text{res}} = N_2(F_p) - N_2^{\text{el}}(F_p) = \underbrace{N_2(F_p)}_{N_Y} - \kappa \underbrace{N_2^{\text{el}}(F_e)}_{N_Y} = N_Y - \kappa N_Y$$



Residual stresses

Elastic solution

$$N_2^{\text{el}}(F) = \frac{F}{1 + 2 \cos^3 \alpha} \quad (\text{linear})$$

Define

$$N_2(F) = \begin{cases} N_2^{\text{el}}(F), & F \leq F_e \\ N_Y, & F > F_e \end{cases}$$

Residual force

$$N_2^{\text{res}} = N_2(F) + N_2^{\text{el}}(-F) = N_2(F) - N_2^{\text{el}}(F), \quad (\text{as long as } N_2^{\text{res}} \geq -N_Y)$$

Limit state

$$N_2^{\text{res}} = N_2(F_p) - N_2^{\text{el}}(F_p) = \underbrace{N_2(F_p)}_{N_Y} - \underbrace{\kappa N_2^{\text{el}}(F_e)}_{N_Y} = N_Y - \kappa N_Y$$

$$N_2^{\text{res}} = N_Y(1 - \kappa)$$

For $\kappa \leq 2$ shakedown occurs, i.e., the structure's response becomes fully elastic.



Life time prediction

High cycle fatigue

Low cycle fatigue



Life time prediction

High cycle fatigue

- Whole process is entirely elastic.
- Crack propagation is the culprit.
- Fatigue strength σ_c may be introduced so that $\sigma_{\max} < \frac{\sigma_c}{k}$.

Low cycle fatigue

- Repeated plastic deformation in cycles.
- If $\kappa \leq 2$ do nothing.
- If $\kappa > 2$, compute $\Delta\epsilon^p$ and use a strain-life method (Coffin-Manson).